

**PRE-BID ADDENDUM****Cuba Township Office Renovation****28000 W Cuba Rd, Barrington, Illinois - 60010**

APA PROJECT #	25-023	DATE ISSUED	10-7-26
ADDENDUM NO.	001	BID DUE	10-7-26
OWNER	Cuba Township	ISSUED BY	APA

This Addendum forms part of Bidding Documents and modifies or clarifies them as described below. Bidders shall acknowledge receipt of this Addendum on the Bid Form.

1. Bidder Question & Answers

Question 1 : Confirm if material testing and Geotech testing are by owner.

Answer 1 : Yes, the testing is complete. Please find the report attached here.

Question 2 : Confirm that existing furniture and furnishings will be removed by owner.

Answer 2 : Yes, Owner is responsible for removal of furniture and furnishings.

Question 3 : Confirm new furniture is by owner.

Answer 3 : Yes, Owner is responsible for new furniture.

Question 4 : Please issue plumbing drawings and specifications.

Answer 4 : Plumbing work is not included in the scope of work for this bid; therefore, plumbing drawings and specifications are not provided.

Question 5 : Please issue sprinkler drawings and specifications.

Answer 5 : Sprinkler work is not included in the scope of work for this bid; therefore, Sprinkler drawings and specifications are not provided.

Question 6 : Please provide soils report.

Answer 6 : Please find the report attached here.

Question 7 : Please confirm that there is no lead-based paint or asbestos present.

Answer 7 : Owner confirms that there no lead-based paint or asbestos present.

Question 8 : Please confirm if video monitor is by owner if not, please provide specifications and in wall backing requirements.

Answer 8 : Video Monitor will be provided by the owner

Question 9 : Please confirm that there is no automatic door operators required.

Answer 9 : No automatic door operators. There is only a magnetic hold open device at door 113 that would require power supply. Refer to door schedule and power plans for details.



Question 10 : Please confirm that there is no window coverings required.

Answer 10 : They are not required

Question 11 : Please provide room finish schedule and confirm existing flooring material to be patched at demoed walls/cabinets in existing building.

Answer 11 : Please see the attached room finish plan. Yes, existing materials must be repaired/patched at demoed walls/cabinets.

Question 12 : For the interior aluminum offices. Are these demountable glass partitions, interior storefronts, hurculite glass doors and glass walls?

Answer 12 : Interior glass partitions would be of demountable glass.

Question 13 : Please confirm if Nova is doing the sprinkler scope of work including the water tank in the basement (per G1.2) direct with the owner or is the General contractor to include the sprinkler work in their bid

Answer 13 : Yes, Nova has a separate contract with the owner and they will cover the sprinkler scope of work

Question 14 : Per the Proposed Occupancy, this is Group B Business. Per the Fire Sprinkler Notes on Sheet A1-2 we are to install a NFPA 13D system. This is not per code. Since this is business, it needs to be NFPA 13. If there is no water, I assuming not because of that area, the tank will need to be bigger. There is also a note from the City's FPD about NFPA13D. Please advise.

Answer 14 : Cuba township building does not fall under lake county supplied water system and does fall within the county's fire hydrant line. It relies on well water and hence its own water system. Based on these facts, BCFPD has issued a letter confirming we can go with NFPA13D system which will allow us to keep the proposed tank size. This approach has been reviewed and approved by the Lake County permit officials.

Question 15 : Per the notes, we are only installing Fire Sprinklers in the following; Conference Room, Office 1, Foyer and the New Storage Room in the Basement? This will not be a code compliant system. Please advise

Answer 15 : To provide sprinkler system only to the new addition, we have separated the existing & new building by a fire wall. This approach has been reviewed and accepted by lake County permit officials.

Question 16 : Please confirm if we can use square corner sign frames in lieu of radius.

Answer 16 : Owner approves the change.

2. Changes to Bidding Documents

None



3. Attachments

Included with this Addendum: Soil & Geotech report, Interior Finish plan

END OF ADDENDUM NO [001]

Bid Form Acknowledgement

The Bidder acknowledges receipt of the Addenda listed below. Enter each addendum number and issue date, including this addendum.

Addendum No	Issue Date
001	10-7-2026



Office: 847-870-0544
Fax: 847-870-0661
us@soilandmaterialconsultants.com
www.soilandmaterialconsultants.com

January 30, 2026
File No. 29166
REVISED

Mr. Christopher Karam
Cuba Township
28000 W. Cuba Road
Barrington, IL 60010

Re: Geotechnical Investigation
28000 W. Cuba Road
Barrington, Illinois

Dear Mr. Karam:

We are submitting our revised report for the subsurface investigation completed at 28000 W. Cuba Road in Cuba Township, Illinois. This revised report includes additional design values.

The investigation was requested to determine current subsurface soil and water conditions at select boring locations. The findings of the field investigation and the results of laboratory testing are intended to assist in the planning, design and construction of proposed site improvements.

PROPOSED IMPROVEMENTS

We understand it is proposed to construct a single-story addition to the northeast side of the existing building. The addition is expected to be supported on shallow depth foundations with interior basement slabs supported on prepared subgrade soils. Finished floor and basement slab elevations of the addition are expected to match those of the existing building. Additional improvements are expected to include underpinning the southern wall of the addition, pavement areas, and related underground improvements.

SCOPE OF THE INVESTIGATION

The field investigation included obtaining 2 borings at the locations requested and as indicated on the enclosed location sketch. The boring locations were established using field taping methods and accuracy. Surface elevations were determined using the temporary benchmark indicated on the location sketch.

We auger drilled the borings to depths of 20.0 feet below existing surface elevations. Soil samples were obtained using a split barrel sampler advanced utilizing an automatic SPT hammer. Soil profiles were determined in the field and soil samples returned to our laboratory for additional testing including determination of moisture content. Cohesive soils obtained by split barrel sampling were tested further to determine dry unit weight and unconfined compressive strength.

The results of all field determinations and laboratory testing are included in summary with this report.

8 W. COLLEGE DR. • SUITE C • ARLINGTON HEIGHTS, IL 60004

SOIL BORINGS • SITE INVESTIGATIONS • PAVEMENT INVESTIGATIONS • GEOTECHNICAL ENGINEERING
TESTING OF • SOIL • ASPHALT • CONCRETE • MORTAR • STEEL

RESULTS OF THE INVESTIGATION

Enclosed are boring logs indicating the soil conditions encountered at each location. Site surface conditions include the existing structure, pavement areas, vegetation, topsoil and fill soil conditions. The topsoil is classified as dark brown to black silt/clay mixtures with traces of roots usually present.

Fill soil conditions were encountered at each boring location. Composition of the fill includes the presence of clay/silt and silt/clay mixtures extending to a depth of 8.5 feet at boring B-1 and to a depth of 4.0 feet at boring B-2. The limits of fill placement were not determined within the scope of this investigation. Larger debris may also be present within the fill but was not encountered during the investigation. The fill soil conditions are found to overlie the apparent natural topsoil at boring B-1 extending to a depth of 6.5 feet.

Underlying natural soil conditions include the presence of cohesive soils. These are classified as very tough to hard clay/silt mixtures with lesser portions of sand and gravel. The upper portion of these soils at boring B-2 were found to be high in moisture content with a value of 32.6% determined. Non-cohesive soils were also encountered as indicated. These include medium-dense silt/clay mixtures a very damp condition. Cobbles and boulders may be present within the site soils at any elevation, although none were encountered while drilling.

The following table summarizes depth ranges below existing grade, the magnitude of soil strength within these ranges and other information:

<u>Boring</u>	<u>Surface Elevation (feet)</u>	<u>Depth Range Below Existing Surface (feet)</u>	<u>Soil Strength (lbs./sq.ft.)</u>	<u>Recorded Water Levels, W.D./A.D. (feet)</u>
1	95.5	1.0 to 2.0	*3,000	dry / dry
		2.0 to 8.5	*1,000	
		8.5 to 17.0	3,000	
2	95.5	1.0 to 6.5	*1,000	dry / dry
		6.5 to 8.0	*2,000	
		8.0 to 9.5	5,000	
		9.5 to 17.0	3,000	

* Not recommended for support of foundations. Deeper foundation depths or foundations supported on coarse crushed stone fill may be needed to reduce the magnitude of long-term total and differential settlement.

It is expected that foundations can be supported on undisturbed natural soils located at any elevation within the depth ranges indicated in the above table, except as noted. Above these depth ranges the soils are not considered able to support foundations, even at reduced design bearing values, due to long-term settlement considerations.

SUBSURFACE WATER

The boring logs and the above table indicate subsurface water was not encountered in the bore holes at the time of the drilling operations and during the period of these readings. It is expected that fluctuations from the water levels recorded will occur over a period of time due to variations in rainfall, temperature, subsurface soil conditions, soil permeability and other factors not evident at the time of the water level measurements.

FOUNDATIONS

Based on the results of this investigation it is our opinion that continuous and isolated footing foundations may be considered for support of building loads. These foundations can be supported on undisturbed natural soils located below all topsoil, debris, unsuitable fill soils, low strength soils and other unsuitable conditions which may be encountered. Soil strength values and the depths at which they are expected to be encountered at these boring locations are indicated in the above table. A net allowable bearing value of 3,000 lbs./sq.ft. is available for design. This value can be used to size foundations for support of structure dead and live loads.

We understand the new foundation will extend approximately 6.25 feet below existing grade. Weak soil conditions are expected to be discovered at design foundation elevations and may require extending the foundation to a deeper elevation. Alternately, removal of the weak soil followed by replacement with properly compacted coarse crushed granular fill (CA01) may be feasible. When removal is approved by the Soil Engineer, the removal of the weak soil should also extend beyond the face of footings and/or piers to a distance at least equal to the depth of fill that will be present beneath the footings and/or piers.

All exterior building foundations should extend at least 42.0 inches below exposed surface elevations to provide adequate protection against uplift due to freezing of the supporting soils. Foundations for unprotected improvements should extend at least 48.0 inches below exposed surface elevations. We recommend providing adequate reinforcing steel in foundation walls and piers to minimize the effects of long-term differential settlement.

DESIGN VALUES

Where applicable, the following average values can be utilized for design:

Fill Soils

Unit Dry Weight (lbs./cu.ft.)	=	115
Unit Wet Weight (lbs./cu.ft.)	=	130
Angle of Internal Friction	=	26°
Active Stress Coefficient, K_a	=	0.39
Passive Stress Coefficient, K_p	=	2.56
At-Rest Stress Coefficient, K_a	=	0.56

Natural Clay/Silt Soils

Unit Dry Weight (lbs./cu.ft.)	= 120
Unit Wet Weight (lbs./cu.ft.)	= 138
Angle of Internal Friction	= 28°
Active Stress Coefficient, K_a	= 0.36
Passive Stress Coefficient, K_p	= 2.77
At-Rest Stress Coefficient, K_a	= 0.53

Factors of Safety are not applied to the above values. All backfill materials placed behind walls are presumed to be drained. Passive Pressures within 4.0 feet of the surface should be omitted due to seasonal weather considerations.

FLOOR SLABS

Floor slabs planned for support on the existing soil conditions are expected to undergo some degree of long-term settlement as the soils consolidate under loading and as they shrink due to desiccation. Slabs may be considered for support on suitable natural soils or on properly placed and compacted fill soils. This is feasible when the soils supporting the slabs are prepared in accordance with the recommendations for Subgrade Soil Preparation. These include the removal of topsoil as well as removal or aeration of underlying high moisture content soils.

DEWATERING

Excavations may require dewatering due to subsurface water seepage and/or surface precipitation. This water can be removed by standard sump and pump operations. Soils exposed at foundation, slab or undercut elevations should not be permitted to become saturated. Loss of bearing strength and stability may occur, requiring additional soil excavation.

Fill soils, non-cohesive soils and others can be unstable when saturated. These soils tend to cave or run when submerged or disturbed. The stability of exposed embankments is minimal to non-existent as confining soil pressures are removed. Proper drainage within excavations is necessary at all times, particularly when excavations extend below anticipated water levels and below saturated soils.

The contractor should be made responsible for designing and constructing stable temporary excavations. Also, the contractor should shore, slope, bench or restrain the sides of the excavations as required to maintain stability of both the excavation sides and bottom. In no case, should the slope, slope heights, or excavation depth exceed those in the local, state, and federal safety regulations.

Permanent dewatering of basement, crawl space and other below grade areas is necessary. The dewatering system should include the provision for peripheral drain tile adjacent to the footings of foundation walls exposed to the interior of the building. We recommend damp-proofing or possibly water-proofing exterior foundation walls exposed to the exterior of the building.

SUBGRADE SOIL PREPARATION

Subgrade soil preparation should be accomplished where needed within the building area prior to excavation for foundations. The procedure in all areas of subgrade supported improvements should include the removal of unsuitable surface conditions including vegetation, topsoil, unsuitable fill soils, significant debris, weak or unstable soils, and other deleterious conditions which may be encountered. Above grade areas should be cut to design subgrade elevations. Exposed subgrade soils should be leveled, compacted and proof-rolled in the presence of the Soil Engineer.

Proof-rolling may reveal areas of unstable soil conditions, requiring additional removal. If the depth of undercut appears to be significant, it may be economical to limit the depth of undercut to that needed to establish adequate support of slabs and remediate weak soil conditions at foundation elevations at the time of foundation construction.

Soft or unstable soil conditions in pavement areas can often be bridged by use of an effective depth of crushed granular material. The placement of the crushed granular bridging material, possibly in conjunction with the use of an appropriate geotextile fabric, should only proceed after review of the proof-roll conditions by the Soil Engineer. Long-term settlement of pavement surfaces may occur locally as the bridged soils desiccate.

Structural fill can be placed on soils prepared to the satisfaction of the Soil Engineer. The fill should be placed in lifts not to exceed 8.0 inches when uncompacted. Each lift should exceed minimum compaction requirements prior to placement of the next lift. We recommend a minimum of 95% compaction based on the modified Proctor test, ASTM D-1557, be achieved within building areas. A minimum of 90% compaction should be achieved beneath exterior improvements such as pavements and sidewalks. Compaction requirements also apply to backfill placement around foundations and within trench excavations located below subgrade supported improvements.

FILL SOURCES

The onsite non-organic soils are generally suitable for reuse as fill. Offsite sources may also be used provided they are approved in advance by the Soil Engineer. Aeration may be necessary to reduce soil moisture content prior to compaction. Soil borrowed from near the surface where seasonal fluctuations in soil moisture content occur may require particular attention. The moisture content of fill soils should be within approximately 3.0% of optimum moisture content as determined by the modified Proctor test for the soils to meet or exceed minimum compaction requirements.

CONCLUSION

The information within this report is intended to provide initial information concerning subsurface soil and water conditions on the site. Variations in subsurface conditions are expected to be present between boring locations due to naturally changing soil conditions. Variations are also expected within areas of disturbed (filled) soil conditions.

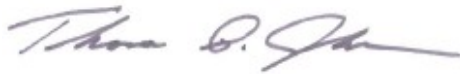
Our understanding of the proposed improvements is based on limited information available to us at the writing of this report. The findings of the investigation and the recommendations presented are not considered applicable to significant changes in the scope of the improvements or applicable to alternate site uses. We recommend that proposed foundation, pavement and grading plans be reviewed by our office to determine if additional considerations are necessary to address anticipated subsurface conditions.

The soils exposed in soil undercut areas should be evaluated for suitability prior to placement of structural fill, as previously indicated in this report. Soils and aggregates placed as structural fill should be tested as the work progresses to verify that minimum compaction requirements have been met. We recommend that soil conditions encountered at foundation elevations be tested to verify the presence of design soil strength prior to concrete placement.

Any questions concerning the information presented in this report should be directed to our office.

Very truly yours,

SOIL AND MATERIAL CONSULTANTS, INC.



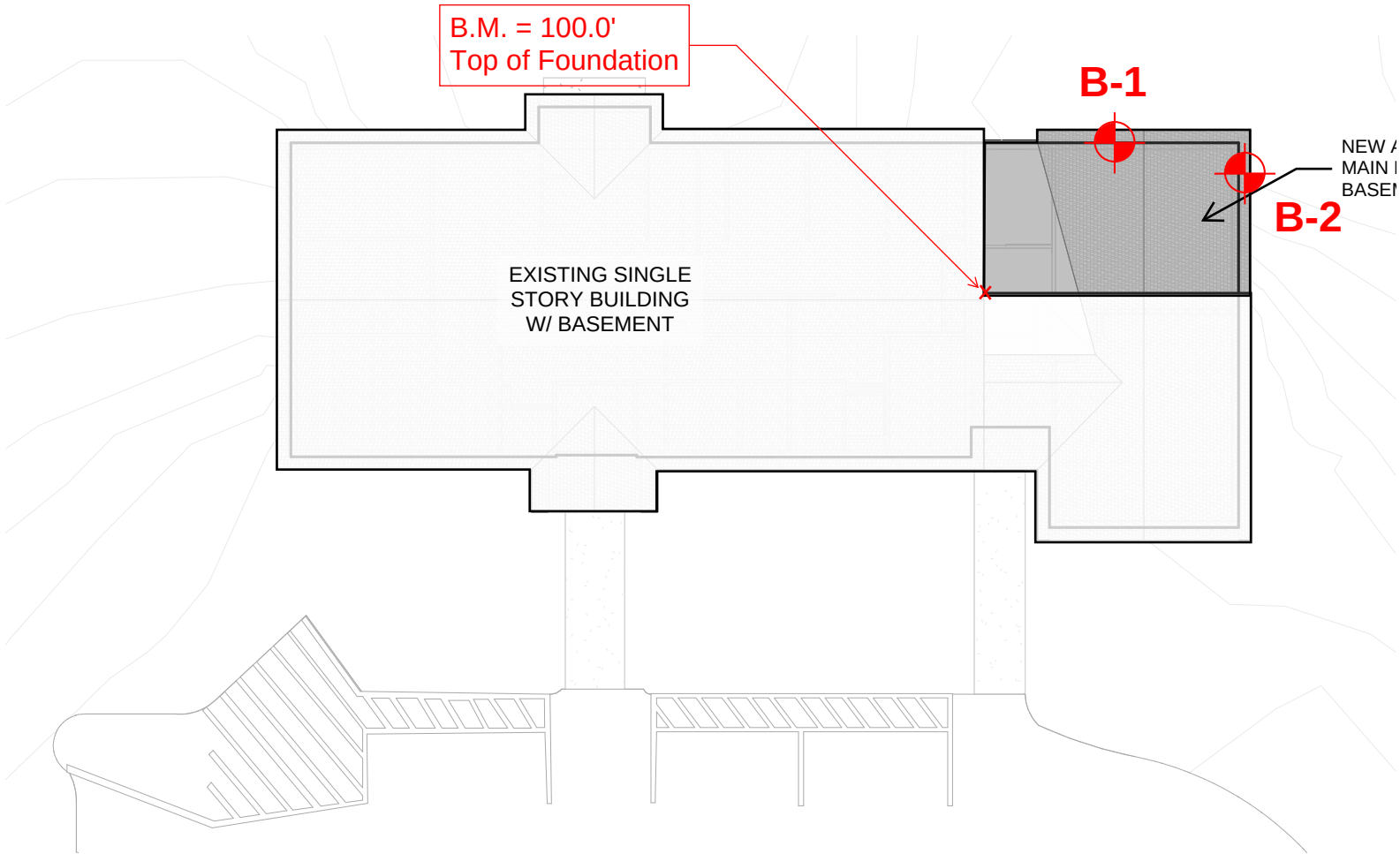
Thomas P. Johnson, P.E.
President



Martin Clarke
Project Engineer

TPJ:MC:ek
Enc.

cc: Mr. Eric Pepa, AIA, NCARB – Allen+Pepa Architects



SMC		SOIL AND MATERIAL CONSULTANTS, INC.	LOCATION SKETCH
Client:	CUBA TOWNSHIP		
Project:	28000 W. CUBA ROAD		
Location:	BARRINGTON, ILLINOIS		
File No.	29166	Date: 12-23-25	Scale: 1" ≈ 20'

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SOIL BORING LOG 1

Logged By: CS Page: 1 of 1

Client: Cuba Township

File No. 29166 Date Drilled: 12/23/25

Reference: 28000 W. Cuba Road
Barrington, IL

Comments:

depth, ft.	Equipment: ☒ D - 25 ☐ D - 50 ☐ Hand Auger ☐ Other	standard penetration	moisture content	dry unit weight lbs./cu.ft.	unconfined compressive strength	○ unconfined compressive strength, tons/sq. ft. ● penetrometer reading, tons/sq. ft. 1.0 2.0 3.0 4.0			
	CLASSIFICATION					× standard penetration "N", blows/ft. △ moisture content, % 10 20 30 40			
	Elevation 95.5' Existing Surface	×	△	×	○				
	Topsoil - Fill - 12.0"								
	Brown-gray clay, some silt, trace sand & gravel, damp, very tough to tough - Fill	12	19.7	110.6	3.4	×	△	●	○
5	Dark brown silt, some clay, trace sand, damp, loose - Fill	7	21.8 25.0	107.2	1.6	×	●	○	△
	Black-dark brown silt, some clay, trace sand, damp, medium dense - Fill	11	24.5			×	△		
10	Brown-gray clay, some silt, trace sand & gravel, damp, hard	17	15.2	118.7	5.7	△	×		5.7
	Gray silt, some clay, trace fine sand, very damp, medium dense	15	15.5 19.2	119.4	4.8	△	×		4.8
15		16	18.2			×			
20	End of Boring	12	17.7			×	△		
25									
30									
35									
40									

Water encountered at dry feet during drilling operations (W.D.)
 Water recorded at dry feet on completion of drilling operations (A.D.)
 Water recorded at feet hours after completion of drilling operations (A.D.)

SOIL BORING LOG 2

Logged By: CS

Page: 1 of 1

Client: Cuba Township

File No. 29166

Date Drilled: 12/23/25

Reference: 28000 W. Cuba Road
Barrington, IL

Comments:

depth, ft.	Equipment: ☒ D - 25 ☐ D - 50 ☐ Hand Auger ☐ Other	standard penetration	moisture content	dry unit weight lbs./cu. ft.	unconfined compressive strength	○ unconfined compressive strength, tons/sq. ft. ● penetrometer reading, tons/sq. ft. 1.0 2.0 3.0 4.0			
	CLASSIFICATION					× standard penetration "N", blows/ft. △ moisture content, % 10 20 30 40			
	Elevation 95.5' Existing Surface	×	△	×	○				
	Dark brown silt, some clay, trace sand & gravel, damp, loose - Fill	7	22.0						
	Brown clay, some silt, trace sand & gravel, damp, very tough - Fill		21.0	102.4	2.2	×	●	△	
5	Black silt, some clay, trace sand & roots, damp, loose (topsoil)	8	32.7			×		△	
	Brown-gray clay, some silt, trace sand & gravel, damp, very tough to hard	9	32.6	88.0	2.4	×	●	△	
10		10	15.8	122.2	4.7	×	△		4.7 ○
	Gray silt, some clay, trace fine sand, very damp, medium dense	21	15.0	123.4	6.9		△	×	6.9 ○
15		11	18.5			×	△		
20	End of Boring	14	18.3			×			
25									
30									
35									
40									

Water encountered at dry feet during drilling operations (W.D.)
 Water recorded at dry feet on completion of drilling operations (A.D.)
 Water recorded at feet hours after completion of drilling operations (A.D.)



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GENERAL NOTES

SAMPLE CLASSIFICATION

Soil sample classification is based on the Unified Soil Classification System, the Standard Practice for Description and Identification Soils (Visual-Manual Procedure), ASTM D-2488, the Standard Test Method for Classification of Soils for Engineering Purposes, ASTM D-2487 (when applicable), and the modifiers noted below.

CONSISTENCY OF COHESIVE SOILS

<u>Term</u>	<u>Qu-tons./sq.ft.</u>	<u>N (unreliable)</u>
Very soft	0.00 – 0.25	0 – 2
Soft	0.26 – 0.49	3 – 4
Stiff	0.50 – 0.99	5 – 8
Tough	1.00 – 1.99	9 – 15
Very Tough	2.00 – 3.99	16 – 30
Hard	4.00 – 7.99	30 +
Very Hard	8.00 +	

RELATIVE DENSITY OF GRANULAR SOILS

<u>Term</u>	<u>N – blows/foot</u>
Very Loose	0 – 4
Loose	5 – 9
Medium Dense	10 – 29
Dense	30 – 49
Very Dense	50 +

IDENTIFICATION AND TERMINOLOGY

<u>Term</u>	<u>Size Range</u>
Boulder	over 8 in.
Cobble	3 in. to 8 in.
Gravel - coarse	1 in. to 3 in.
- medium	3/8 in. to 1 in.
- fine	#4 sieve to 3/8 in.
Sand - coarse	#10 sieve to #4 sieve
- medium	#40 sieve to #10 sieve
- fine	#200 sieve to #40 sieve
Silt	0.002 mm to #200 sieve
Clay	smaller than 0.002mm

Modifying Term Percent by Weight

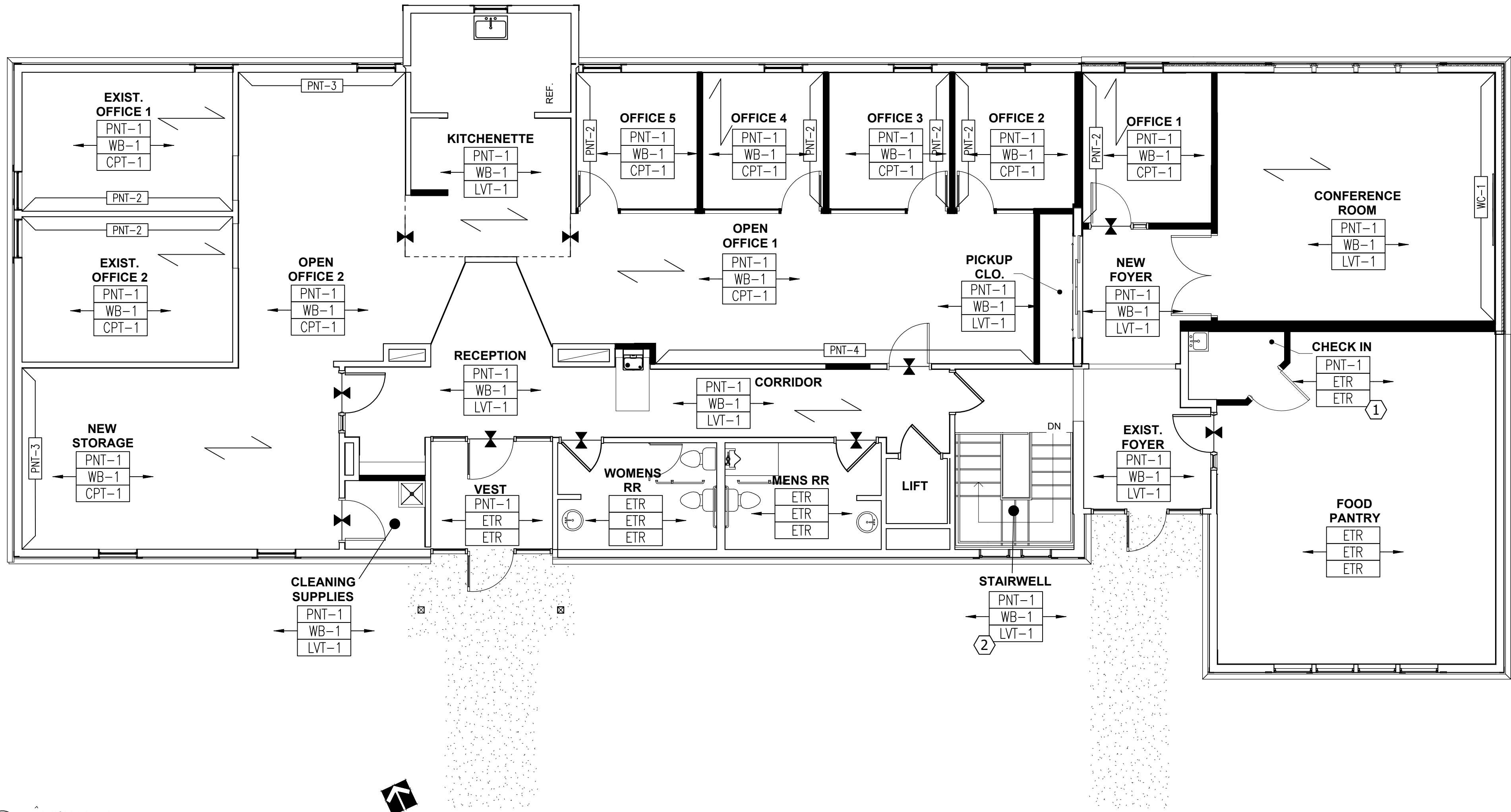
Trace	1 – 10
Little	11 – 20
Some	21 – 35
And	36 – 50

Moisture Content

Dry
Damp
Very Damp
Saturated

DRILLING, SAMPLING & SOIL PROPERTY SYMBOLS

CF	- Continuous Flight Auger
HS	- Hollow Stem Auger
HA	- Hand Auger
RD	- Rotary Drilling
AX	- Rock Core, 1-3/16 in. diameter
BX	- Rock Core, 1-5/8 in. diameter
NX	- Rock Core, 2-1/8 in. diameter
S	- Sample Number
T	- Type of Sample
J	- Jar
AS	- Auger Sample
SS	- Split Spoon (2 in. O.D. with 1-3/8 in. I.D.)
ST	- Shelby Tube (2 in. O.D. w/ith 1-7/8 in. I. D.)
R	- Recovery Length, in.
B	- Blows/6 in. interval, Standard Penetration Test (SPT)
N	- Blows/foot to drive 2 in. O.D. split-spoon sampler with 140 lb. hammer falling 30 in., (STP)
Pen.	- Pocket Penetrometer readings, tons/sq.ft.
W	- Water Content, % dry weight
Uw	- Dry Unit Weight of soil, lbs./cu.ft.
Qu	- Unconfined Compressive Strength, tons/sq.ft.
Str	- % Strain at Qu.
WL	- Water Level
WD	- While Drilling
AD	- After Drilling
DCI	- Dry Cave-in.
WCI	- Wet Cave-in.
LL	- Liquid Limit, %
PL	- Plastic Limit, %
PI	- Plasticity Index (LL-PL)
LI	- Liquidity Index [(W-PL)/PI]



1 FINISH PLAN
SCALE: 1/8"=1'-0"

GENERAL ABBREVIATION KEY:
AFF - ABOVE FINISHED FLOOR
ETR - EXISTING TO REMAIN
NTS - NOT TO SCALE
OC - ON CENTER
UNO - UNLESS NOTED OTHERWISE

SYMBOL KEY:
FLOOR TRANSITION
FINISH TYPE
KEYNOTE
AREA NOT IN CONTRACT

FINISH PLAN GENERAL NOTES:
1. ALL WALLS TO BE PAINTED PNT-1 WITH WB-1 WALLBASE, UNO.
2. ALL DOOR FRAMES TO BE PAINTED PNT-1
3. PAINT FINISHES ARE TO BE AS FOLLOWS:
GENERAL WALLS TO RECEIVE EGGSHELL FINISH, UNO
CEILINGS TO RECEIVE FLAT FINISH
DOORS AND DOOR FRAMES TO RECEIVE SEMI-GLOSS FINISH
4. CPT-1 TO BE INSTALLED ASHLAR THROUGHOUT. SEE FINISH PLAN FOR INSTALLATION DIRECTION.
5. LVT-1 TO BE INSTALLED ASHLAR. SEE FINISH PLAN FOR INSTALLATION DIRECTION.
6. ANY WOOD DOORS - TO BE STAINED TO MATCH WALNUT FINISH (TO BE APPROVED BY INTERIOR DESIGNER)
7. GLASS WALL FRAMING TO BE SILVER FINISH

* FLOORING MATERIAL & INSTALL - PROVIDED BY RIEKE INTERIORS
* WALLCOVERING MATERIAL & INSTALL - PROVIDED BY RIEKE INTERIORS

1 PATCH WITH EXISTING LVT IF NECESSARY
(LIFEPROOF/ WALTON OAK VINYL PLANK) 7" X 48"
2 PROVIDE RUBBER NOSING FOR STAIRS

FINISH LEGEND				
MATERIAL TYPE	TAG	MANUFACTURER	PRODUCT/STYLE/COLOR	NOTES
CARPET TILE	CPT-1	BENTLEY	STYLE: SERVE IT UP COLOR: JULIENNE 801542	GENERAL CARPET SIZE: 18" X 36" INSTALL PATTERN: ASHLAR
LUXURY VINYL	LVT-1	MOHAWK	COLLECTION: LIVING LOCAL STYLE: WOODS 4.5 C2120 COLOR: 868 YOSEMITE	GENERAL LVT SIZE: 9.25" X 59" WEAR LAYER: 20 MIL THICKNESS: 4.5 MM INSTALL PATTERN: ASHLAR
VINYL WALLBASE	WB-1	TARKETT	STYLE: TRADITIONAL WALL BASE 63 COLOR: BURNT UMBER B	INSTALL ON ALL WALLS AS NOTED
VINYL FLOOR TRANSITION STRIP	N/A	SCHLUTER	COLOR: SCHIENE #AE30	SLIM LINE TRANSITION IN DIMENSIONS REQUIRED PER FLOORING THICKNESS AS DETERMINED BY FLOORING CONTRACTOR. "FLOOR TRANSITION SYMBOL" REPRESENTS TRANSITION LOCATION ON FINISH PLAN.
PAINT	PNT-1	BENJAMIN MOORE	COLOR: CHANTILLY LACE OC-65	GENERAL WALL PAINT AND GENERAL DOOR TRIM EGGSHELL FINISH
PAINT	PNT-2	BENJAMIN MOORE	COLOR: SEAPEARL OC-19	ACCENT WALL PAINT. SEE FINISH PLAN FOR LOCATIONS. EGGSHELL FINISH
PAINT	PNT-3	BENJAMIN MOORE	COLOR: AMSTERDAM AF-550	ACCENT WALL PAINT SEE FINISH PLAN FOR LOCATIONS. EGGSHELL FINISH
PAINT	PNT-4	BENJAMIN MOORE	COLOR: GREAT BARRINGTON GREEN HC-122	ACCENT WALL PAINT SEE FINISH PLAN FOR LOCATIONS. EGGSHELL FINISH
WALLCOVERING	WC-1		PROVIDED BY RIEKE INTERIORS	INSTALLED BY RIEKE INTERIORS
WALLCOVERING	WC-2		PROVIDED BY RIEKE INTERIORS	INSTALLED BY RIEKE INTERIORS

ROOM FINISH SCHEDULE - FIRST FLOOR					
ROOM NUMBER	ROOM NAME	FLOOR FINISH	WALLBASE	WALL FINISH	NOTES
	VESTIBULE	ETR	ETR	PNT-1	
	RECEPTION	LVT-1	WB-1	PNT-1	
	CLEANING SUPPLIES	LVT-1	WB-1	PNT-1	
	NEW STORAGE	CPT-1	WB-1	PNT-1/ PNT-3	SEE FINISH PLAN FOR ACCENT LOCATION
	EXIST OFFICE 1	CPT-1	WB-1	PNT-1/ PNT-2	SEE FINISH PLAN FOR ACCENT LOCATION
	EXIST OFFICE 2	CPT-1	WB-1	PNT-1/ PNT-2	SEE FINISH PLAN FOR ACCENT LOCATION
	OPEN OFFICE 2	CPT-1	WB-1	PNT-1/ PNT-3	SEE FINISH PLAN FOR ACCENT LOCATION
	KITCHENETTE	LVT-1	WB-1	PNT-1	
	OPEN OFFICE 1	CPT-1	WB-1	PNT-1/ PNT-4	SEE FINISH PLAN FOR ACCENT LOCATION
	OFFICE 1	CPT-1	WB-1	PNT-1/ PNT-2	SEE FINISH PLAN FOR ACCENT LOCATION
	OFFICE 2	CPT-1	WB-1	PNT-1/ PNT-2	SEE FINISH PLAN FOR ACCENT LOCATION
	OFFICE 3	CPT-1	WB-1	PNT-1/ PNT-2	SEE FINISH PLAN FOR ACCENT LOCATION
	OFFICE 4	CPT-1	WB-1	PNT-1/ PNT-2	SEE FINISH PLAN FOR ACCENT LOCATION
	OFFICE 5	CPT-1	WB-1	PNT-1/ PNT-2	SEE FINISH PLAN FOR ACCENT LOCATION
	WOMEN'S RR	ETR	ETR	ETR	
	MEN'S RR	ETR	ETR	ETR	
	CORRIDOR	LVT-1	WB-1	PNT-1	
	STAIRWELL	LVT-1	WB-1	PNT-1	
	PICKUP CLOSET	LVT-1	WB-1	PNT-1	
	NEW FOYER	LVT-1	WB-1	PNT-1	
	EXIST FOYER	LVT-1	WB-1	PNT-1	
	FOOD PANTRY	ETR	ETR	PNT-1/ WC-2	SEE FINISH PLAN FOR ACCENT LOCATION
	CHECK IN	ETR	ETR	PNT-1	
	CONFERENCE ROOM	LVT-1	WB-1	PNT-1/ WC-1	SEE FINISH PLAN FOR ACCENT LOCATION



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IN SIGNING THIS DOCUMENT, YOU ARE IN AGREEMENT THAT THIS DOCUMENT MEETS YOUR SPECIFICATIONS. PLEASE NOTE THAT CHANGES TO ANY PROJECT ELEMENTS AFTER SIGN-OFF MAY BE SUBJECT TO ADDITIONAL CHARGES.

SIGNATURE:

CLIENT NAME:

TITLE:

DATE:

1	1.30.26	FINISH PLAN
	1.23.26	FINISH PLAN
NO.	DATE	DESCRIPTION
REVISION LOG		

CUBA TOWNSHIP
28000 W. CUBA ROAD
BARRINGTON, IL 60010

PROJECT
ADDRESS:



FIRST FLOOR
FINISH PLAN

ID-104.00